Lesson 1: Application of Reynolds Transport Theorem (Flow in a pipe)

Reynolds theorem for an arbitrary (scalar or vector) variable β .

$$\frac{d}{dt} \int_{S(t)} \beta \rho dV = \frac{\partial}{\partial t} \int_{CV} \beta \rho dV + \int_{CS} \beta \rho \vec{v} d\vec{A}$$
(1)
$$d\vec{A} = \vec{n} dA$$



a) Continuity

Take $B = m, \beta = \frac{dB}{dm} = 1$ in (1)

$$\int_{CV} \beta \rho dV = \int_{CV} \rho dV = m = \text{mass}$$

Physical Law: Conservation of mass

$$\frac{dm}{dt} = 0$$
$$0 = \frac{\partial}{\partial t} \int_{CV} \rho dV + \int_{CS} \rho \vec{v} d\vec{A}$$

V=volume bounded by C.S.

$$0 = \rho \frac{\partial V}{\partial t} + \int_{CS} \rho \vec{v} d\vec{A}$$

For a pipe: on the lateral walls $\vec{v} \perp \vec{A}$, so $\vec{v}d\vec{A} = 0$

$$\vec{n} \xleftarrow{1}{V_1} \xleftarrow{n}{V_1}$$

$$\int_{CS} \rho \vec{v} d\vec{A} = \int_{1} \rho \vec{v} d\vec{A} + \int_{2} \rho \vec{v} d\vec{A} = -\rho V_1 A_1 + \rho V_2 A_2$$
$$\Rightarrow -\frac{\partial V}{\partial t} = -V_1 A_1 + V_2 A_2 = -Q_1(t) + Q_2(t)$$

For the case V(t) = const. (rigid wall)

$$Q_2(t) - Q_1(t) = 0 \Rightarrow Q_1 = Q_2$$

b) Momentum

Physical Law: Newton's second law of motion

$$\sum \vec{F} = m\vec{a} = \frac{d}{dt}(m\vec{v}) = \frac{d}{dt} \int_{CV} \rho \vec{v} dV$$

Take
$$B = m\vec{v}$$
, $\beta = \frac{dB}{dm} = \vec{v}$ in (1)

$$\sum \vec{F} = \frac{\partial}{\partial t} \int_{CV} \rho \vec{v} dV + \int_{CS} \rho \vec{v} \left(\vec{v} d\vec{A} \right)$$
⁽²⁾

If flow is steady, i.e. $\frac{\partial}{\partial t} = 0$

Apply (2) along *x*-axis (streamwise direction in a pipe)

$$\sum F_x = \int_{CS} \rho v_x \left(\vec{v} d\vec{A} \right) \tag{3}$$

Application: shear stress distribution in a circular pipe of uniform crosssection inclined at an angle θ with horizontal

Consider steady flow in a pipe of radius R



Consider cylindrical volume element of radius r < R and length Δx .

Assuming streamlines are parallel to the pipe centerline for any cylindrical CV, A1 = A2 = A, V1 = V2 = Q/A. From (3) $E_{-} = E_{-} = W_{-} = E_{-} = oV_{-} (-V_{-}A_{-}) + oV_{-} (V_{-}A_{-}) = 0$ (4)

$$F_{p1} - F_{p2} - W_x - F_f = \rho V_1(-V_1 A_1) + \rho V_2(V_2 A_2) = 0$$
(4)

Use

 $p_{1} = p(x)$ $p_{2} = p(x + \Delta x) = p(x) + \frac{dp}{dx}\Delta x = p_{1} + \frac{dp}{dx}\Delta x$ $\sin\theta = \frac{dz}{dx}$

$$F_{p1} - F_{p2} = p_1 A_1 - p_2 A_2 = -\frac{dp}{dx} A \Delta x$$
$$F_f = \tau(r) \cdot (lateral \ surface \ of \ CV) = \tau(2\pi r \Delta x)$$
$$W_x = W \sin\theta = \gamma V \sin\theta = \gamma A \Delta x \sin\theta$$

Plug in (4), divide by $(\Delta x \cdot \pi r)$

$$\tau(r) = \frac{\gamma r}{2} \left[-\frac{d}{dx} \left(\frac{p}{r} + z \right) \right]$$

For r = 0, $\tau(r = 0) = 0$.

For
$$r = 0$$
, $\tau(r = R) = \tau_0 = \frac{\gamma R}{2} \left[-\frac{d}{dx} \left(\frac{p}{r} + z \right) \right]$.

Also for a circular pipe as $V_1 = V_2$

$$dH = d\left(\frac{p}{r} + z + \frac{V^2}{2g}\right) = d\left(\frac{p}{r} + z\right), H = \text{total energy (head)}$$

$$\tau_0 = \frac{\gamma R}{2} \left[-\frac{dH}{dx} \right] \Rightarrow \frac{2\tau_0}{\gamma R} = -\frac{dH}{dx} = -\frac{\Delta H_{12}}{\Delta x_{12}} = \frac{h_{f12}}{\Delta x_{12}}$$

 h_f = head loss due to friction forces as difference in total head (energy) between two sections. i.e. the head (energy) lost between the same two sections

$$h_f = \frac{2\tau_0}{\gamma R} \Delta x = \frac{4\tau_0}{\gamma D} \Delta x \Rightarrow h_f \sim \tau_0$$

c) Energy

Energy equation for steady incompressible flow between two sections

$$z_1 + \frac{p_1}{\gamma} + \alpha_1 \frac{V_1^2}{2g} + h_p = z_2 + \frac{p_2}{\gamma} + \alpha_2 \frac{V_2^2}{2g} + h_t + h_{f12}$$

z = elevation above datum of section centroid

 $\alpha = \text{kinetic energy correction factor} = \frac{\int_A u^3 dA}{V^3 A}$, where *V* is the averaged velocity and $V = \frac{\int_A u dA}{VA}$.

 h_p = head added by pumps

 h_t = head removed by turbine

 h_{f12} = head loss due to friction

If flow in the section is uniform $\alpha = 1$.



For laminar (parabolic) profile $\alpha = 2$.

For turbulent (logarithmic or power low) profile $\alpha = 1.03-1.06$.

In many applications of turbulent flow in pipe, assume $\alpha = 1$.

For turbulent flow in a rectangular open channel $\alpha = 1.5-2$.