

海岸动力地貌学

第四章 海岸动力过程

波浪边界层及底床剪应力介绍

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导读

海浪是塑造海岸地貌过程中最积极、最活跃的动力因素。它产生的巨大能量对海岸或海岸建筑物产生很强的冲击力,在近岸物质搬运和堆积方面也有重要作用。潮汐引起的海水周期性升降运动以及随之产生的海水水平方向运动,对塑造海岸地貌也有重要影响,其他的各种动力在不同时空尺度上对海岸也有著或大或小的影响。



Review of boundary layer theory

What is a *boundary layer*?

A boundary layer is the layer of fluid in the immediate vicinity of a bounding surface where the effects of viscosity are significant.







Prandtl/Blasius Boundary Layer Solution

For steady, two-dimensional laminar flows with negligible gravitational effects:





Prandtl/Blasius Boundary Layer Solution



At the edge of B.L.:

$$u_{0}\frac{\partial y_{0}}{\partial x} + v_{0}\frac{\partial y_{0}}{\partial y} = -\frac{1}{\rho}\frac{\partial p_{0}}{\partial x} + v\left(\frac{\partial^{2}y_{0}}{\partial x^{2}} + \frac{\partial^{2}u_{0}}{\partial y^{2}}\right) \qquad \eta = \frac{y}{\delta(x)} = y\sqrt{\frac{U}{2\nu x}} \qquad \psi = \sqrt{2\nu U x}f(\eta)$$
$$u(x, y) = \frac{\partial \psi}{\partial y} = Uf'(\eta)$$
$$u(x, y) = -\frac{\partial \psi}{\partial x} = \sqrt{\frac{\nu U}{2x}}(\eta f' - f)$$
$$u\frac{\partial u}{\partial x} + v\frac{\partial u}{\partial y} = v\frac{\partial^{2}u}{\partial y^{2}} \qquad f''' + f''f = 0$$

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n:

sed:



Law of the wall (Log-law region)



Log-law region

n:
$$u = \frac{u_*}{\kappa} \ln\left(\frac{z}{z_0}\right), \tau = \rho u_*^2$$
 $\tau = \mu \frac{\partial u}{\partial z}\Big|_{z=0}$

*u*_{*}: friction velocity; z_0 : the distance from the boundary to the location of zero velocity; *τ*: bottom (wall) shear stress; *κ*:the Von Kármán constant (=0.40~0.41)



Law of the wall (Log-law region)

Log-law region:

$$u = \frac{u_*}{\kappa} \ln\left(\frac{z}{z_0}\right)$$



Smooth Turbulent Flow

$$\frac{u_*k_s}{v} < 5, z_0 = \frac{v}{9u_*}$$
$$\frac{u}{u_*} = 5.75 \log\left(\frac{u_*z}{v}\right) + 5.5$$



Rough Turbulent Flow

$$\frac{u_*k_s}{v} > 70 - 100, z_0 = \frac{k_s}{30}$$
$$\frac{u}{u_*} = 5.75 \log\left(\frac{30z}{k_s}\right)$$

 $k_s \cong 3d_{90}$ (Julie, 1998)



A) Small-amplitude (linear, Airy) wave theory (Airy, 1844)

 $\zeta = a \cos \theta$: surface elevation/displacement a = H/2: wave amplitude; $\theta = kx - \sigma t$: phase angle $k = 2\pi/L$: wave number; $\sigma = 2\pi/T = 2\pi f$: angular frequency $\sigma^2 = ab \tanh(bb)$ dispersion relation

 $\sigma^2 = gk \tanh(kh)$: dispersion relation

Wave celerity (phase velocity)

Wavelength
$$L = \frac{gT^2}{2\pi} \tanh\left(\frac{2\pi h}{L}\right), \quad C = \frac{gT}{2\pi} \tanh\left(\frac{2\pi h}{L}\right) = \sqrt{\frac{g}{k}} \tanh\left(kh\right)$$

 $u = a\sigma \frac{\cosh\left[k\left(h+z\right)\right]}{\sinh\left(kh\right)} \cos\theta$ Water particle horizontal velocity
 $w = a\sigma \frac{\sinh\left[k\left(h+z\right)\right]}{\sinh\left(kh\right)} \sin\theta$ Water particle vertical velocity



Water particle velocity in a progressive wave



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Definition sketch for wave-induced orbital motion and boundary layer flow

Orbital velocity at the outer edge of boundary layer (Horizontal harmonic oscillation)

$$u_b \equiv u\Big|_{z=-h} = \frac{\pi H}{T} \frac{1}{\sinh kh} \cos \theta = \hat{u}_b \cos \theta, \ \theta \equiv \sigma t - kx$$
 (phase function/angle)

Near bottom velocity amplitude

$$\hat{u}_b \equiv \frac{\pi H}{T \sinh kh}, \sigma = \frac{2\pi}{T}, a = \frac{H}{2} \Longrightarrow \hat{u}_b = \frac{a\sigma}{\sinh kh} = \sigma a_m, \quad a_m \equiv \frac{d_0}{2} = \frac{a}{\sinh kh}$$

 a_m : near bottom orbital amplitude; d_o : orbital diameter (excursion(偏移) length)



Laminar Oscillatory Boundary Layer (small \hat{u}_b)

Navier-Stokes' momentum equation in the *x*-direction





$$u = \Re \left[f(z)e^{i\theta} \right]; e^{i\theta} = \cos\theta + i\sin\theta; \Re: \text{real part}; f(z): \text{complex}$$

Governing Eq.
$$\longrightarrow \quad \frac{d^2 f}{dz^2} - \left(\frac{i\sigma}{v}\right)f = -\left(\frac{i\sigma}{v}\right)\hat{u}_b$$

B.C.
$$f = 0(z = 0); f \rightarrow \hat{u}_b(z \rightarrow \infty)$$

Particular solution: $f = \hat{u}_b$

General solution: $f = Ae^{\pm Dz}$ where $D^2 \equiv i\sigma/\nu \rightarrow D = \sqrt{i\sigma/\nu} = (1+i)\beta$, $\beta \equiv \sqrt{\sigma/2\nu}$

f: to be bounded at $z \to \infty$ \implies $f = \hat{u}_b + Ae^{-(1+i)\beta z}$

B.C. $A = -\hat{u}_b$ $f = \hat{u}_b \left(1 - e^{-(1+i)\varsigma}\right)$ $\zeta = \beta z = \frac{z}{\delta_l}, \quad \delta_l = \frac{1}{\beta} = \sqrt{\frac{2v}{\sigma}} = \sqrt{\frac{vT}{\pi}}$ **Solution** $u = \hat{u}_b \Re\left[e^{i\theta} - e^{i\theta - (1+i)\varsigma}\right]$ $= \hat{u}_b \left[\cos\theta - e^{-\varsigma}\cos\left(\theta - \varsigma\right)\right]$

5/15/2019 Stokes layer thickness, become e⁻¹





Variation of *u* in a laminar oscillatory boundary layer



Velocity amplitude \hat{u} and phase shift ϵ

 $u = \hat{u} \cos\left(\theta + \varepsilon\right)$ $\hat{u} = \hat{u}_b \sqrt{1 - 2e^{-\varsigma} \cos \varsigma} + e^{-2\varsigma}$ $\varepsilon = \tan^{-1} \left[\frac{\sin \varsigma}{e^{\varsigma} - \cos \varsigma}\right]$

Boundary layer thickness δ

$$\frac{\hat{u} - \hat{u}_b}{\hat{u}_b} \cong 1\% \rightarrow \delta = 4.1\delta_l = 4.1\sqrt{\frac{\nu T}{\pi}}$$
$$\delta \cong 2.3 \text{mm}(T = 1s), 7.3 \text{mm}(T = 10s)$$

Amplitude and phase shift of *u* in a laminar wave boundary layer





Bottom friction (Bottom shear stress)

$$\tau_{b} = \mu \frac{\partial u}{\partial z}\Big|_{z=0} = \mu \hat{u}_{b} \left(\frac{\cos\theta}{\delta_{l}} - \frac{\sin\theta}{\delta_{l}}\right) = \sqrt{2}\mu \frac{\hat{u}_{b}}{\delta_{l}} \cos\left(\theta + \frac{\pi}{4}\right) = \hat{\tau}_{b} \cos\left(\theta + \frac{\pi}{4}\right); \quad \mu = \rho \nu$$

 $\hat{\tau}_{_b} \propto \hat{u}_{_b}$ with phase lead of π/4

Define "wave friction factor" f_w as $\hat{\tau}_b = \frac{1}{2} \rho f_w \hat{u}_b^2$

$$f_{w} = \frac{2\hat{\tau}_{b}}{\rho\hat{u}_{b}^{2}} = \frac{2\sqrt{2}\nu}{\hat{u}_{b}\delta_{l}} = \frac{2\sqrt{2}}{R_{\delta}} = \frac{2}{\sqrt{R_{a}}} \qquad R_{\delta} \equiv \frac{\hat{u}_{b}\delta_{l}}{\nu}, \quad R_{a} \equiv \frac{\hat{u}_{b}a_{m}}{\nu} = \frac{R_{\delta}^{2}}{2}$$
$$a_{m} = \frac{\hat{u}_{b}}{\sigma}, \delta_{l} = \sqrt{\frac{\nu T}{\pi}} \qquad a_{m} \equiv \frac{d_{0}}{2} = \frac{a}{\sinh kh}$$



Classification of Wave Boundary Layer

1) Laminar boundary layer

Reynolds number R_a

Approximately $R_a > 10^4 =>$ Laminar to Turbulent

< 5 < $\frac{\hat{u}_*k_s}{v}$ < 70 (Transition)

2) Turbulent boundary layer

- a) Smooth turbulent boundary layer
- b) Rough turbulent boundary layer

 $k_s = 30z_0$ Nikuradse equivalent roughness, z_0 : Roughness length

 $\hat{u}_* \equiv \sqrt{\hat{\tau}_b / \rho}$ Amplitude of friction velocity

Governing Equation

$$\frac{\partial u}{\partial t} = \frac{\partial u_b}{\partial t} + \frac{\partial}{\partial z} \left(\frac{\tau}{\rho}\right)$$

$$\tau = -\rho \overline{u'w'}$$

Reynolds shear stress





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Turbulent Wave Boundary layer and Friction Laws

(1) Jonsson (1966, ICCE, 127): analogy to the steady flow

A) Rough turbulent



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$$\begin{array}{c}
\overbrace{} \underbrace{ \begin{array}{c} \overbrace{} \\ \overbrace{OCEAN COLL} \end{array}}_{OCEAN COLL} \underbrace{ dt}_{dt} \left(\int_{a(t)}^{b(t)} f(x,t) \, dx \right) = \int_{a(t)}^{b(t)} \frac{\partial f}{\partial t} \, dx + f(b(t),t) \cdot b'(t) - f(a(t),t) \cdot a'(t) \\ \overbrace{OCEAN COLL} \end{array} \\
\begin{array}{c} \overbrace{} \\ \underbrace{ \begin{array}{c} \\ \\ \\ \\ \end{array}}_{OCEAN COLL} \underbrace{ dt}_{dt} \left(\int_{a(t)}^{b(t)} f(x,t) \, dx \right) = \int_{a(t)}^{b(t)} \frac{\partial f}{\partial t} \, dx + f(b(t),t) \cdot b'(t) - f(a(t),t) \cdot a'(t) \\ \overbrace{} \\ \underbrace{ \begin{array}{c} \\ \\ \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \end{array}}_{loc} \underbrace{ \begin{array}{c} \\ \end{array}}_{loc} \underbrace{$$

Representative boundary layer thickness $\delta_T = \delta_l(\text{at } \sigma t = 0, \text{ when } u_b(t) = \hat{u}_b)$

$$\hat{\mu}_{*}\delta_{T} = \frac{\kappa^{3}}{2} \frac{\hat{u}_{b}^{2}}{\ln^{2}(\delta_{T}/z_{0})} \frac{\pi}{2\sigma} = \frac{\pi\kappa^{3}}{4} \frac{\hat{u}_{b}a_{m}}{\ln^{2}(\delta_{T}/z_{0})}$$

$$\hat{u}_{b} = a_{m}\sigma$$
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Implicit formula

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$$\frac{\delta_{T}}{z_{0}} \ln\left(\frac{\delta_{T}}{z_{0}}\right) = \frac{\pi\kappa^{2}}{4} \frac{a_{m}}{z_{0}} \implies \ln\left(\frac{\delta_{T}}{z_{0}}\right) + \ln\left[\ln\left(\frac{\delta_{T}}{z_{0}}\right)\right] = \ln\left(\frac{\pi\kappa^{2}}{4}\right) + \ln\left(\frac{a_{m}}{z_{0}}\right)$$
In case of friction factor
$$\Rightarrow f_{w} = \frac{2}{\rho} \frac{\hat{\tau}_{b}}{\hat{u}_{b}^{2}} = 2\left(\frac{\hat{u}_{*}}{\hat{u}_{b}}\right)^{2} = \frac{2\kappa^{2}}{\ln^{2}(\delta_{T}/z_{0})} \implies \ln\left(\frac{\delta_{T}}{z_{0}}\right) = \frac{\sqrt{2}\kappa}{\sqrt{f_{w}}}$$

$$\frac{\sqrt{2}\kappa}{\sqrt{f_{w}}} + \ln\left(\frac{\sqrt{2}\kappa}{\sqrt{f_{w}}}\right) = \ln\left(\frac{\pi\kappa^{2}}{4}\right) + \ln\left(\frac{30a_{m}}{k_{s}}\right)$$

$$\frac{\sqrt{2}\kappa}{\sqrt{f_{w}}} + \frac{1}{4\sqrt{2}\kappa}\ln\left(\frac{\sqrt{2}\kappa}{\sqrt{f_{w}}}\right) = \frac{1}{4\sqrt{2}\kappa}\ln\left(\frac{\pi\kappa^{2}}{4}\times30\right) + \frac{1}{4\sqrt{2}\kappa}\ln\left(\frac{a_{m}}{k_{s}}\right)$$

$$\frac{1}{4\sqrt{f_{w}}} + \log_{10}\frac{1}{4\sqrt{f_{w}}} = \log_{10}\left(\frac{a_{m}}{k_{s}}\right) + 0.22$$
Modification based on measurements (through only few data)
cf. Bagnold's data, f_{w}=0.30 (a_{m}/k_{s}<1.57)

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Explicit approximation to Jonsson (1966) for rough turbulence case, Swart (1974)

$$\begin{cases} f_w = \exp\left[-5.997 + 5.213 \left(\frac{k_s}{a_m}\right)^{0.194}\right] & \frac{k_s}{a_m} < 0.63 \\ f_w = 0.3 & \frac{k_s}{a_m} \ge 0.63 \end{cases}$$

B) Smooth turbulent

Replace z_0 by a parameter, i.e., the viscous sublayer thickness $\delta_v \equiv 11.6v/\hat{u}_*$

Namely,
$$z_0 = \frac{k_s}{30} = \frac{\delta_v}{105} \rightarrow k_s = \frac{3.33v}{\hat{u}_*} = \frac{3.33v}{\sqrt{f_w/2}\hat{u}_b}$$
 $R_a = \frac{\hat{u}_b a_m}{v}$
 $\frac{1}{4\sqrt{f_w}} + 2\log_{10}\frac{1}{4\sqrt{f_w}} = \log_{10}R_a - 1.36$ $z_0 = \frac{k_s}{30} = \frac{\delta_v}{105} \rightarrow k_s = \frac{3.33v}{\hat{u}_*} = \frac{3.33v}{\sqrt{f_w/2}\hat{u}_b}$
Modification
 $\frac{1}{4\sqrt{f_w}} + 2\log_{10}\frac{1}{4\sqrt{f_w}} = \log_{10}R_a - 1.55$ Explicit form $f_w = 0.09R_a^{-0.2}$





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(2) Kajiura (1968, Bull. Earthquake Res. Inst.. Univ. of Tokyo, Vol 46, 75)

Gov. Eq.
$$\Longrightarrow \quad \frac{\partial u}{\partial t} = \frac{\partial u_b}{\partial t} + \frac{\partial}{\partial z} \left(\frac{\tau}{\rho} \right) = \frac{\partial u_b}{\partial t} + \frac{\partial}{\partial z} \left(v_e \frac{\partial u}{\partial z} \right)$$

Kinematic eddy viscosity v_e : Three-layer model

A) Smooth turbulent

$$v_{e} = \begin{cases} v & (0 \le z \le \delta_{i}) : \text{inner layer} \\ \kappa \hat{u}_{*}z & (\delta_{i} \le z \le \delta_{o}) : \text{overlap layer} \\ K \hat{u}_{b} \delta^{*} & (\delta_{o} \le z) : \text{outer layer} \end{cases}$$



Schematic of v_e distribution

 δ_i Inner layer (viscous sublayer) thickness $= 12 \nu / \hat{u}_* \cong \delta_V$ δ_o wall layer thickness $\leftarrow \kappa \hat{u}_* \delta_o = K \hat{u}_b \delta^*$ $K = 0.02; \ \delta^* \equiv \operatorname{Amp}\left(\int_0^\infty \frac{u_b - u}{\hat{u}_b} dz\right)$: displacement thickness

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B) Rough turbulent

 $v_{e} = \begin{cases} 0.185\kappa \hat{u}_{*}k_{s} & (0 \le z \le k_{s}/2) : \text{inner layer} \\ \kappa \hat{u}_{*}z & (k_{s}/2 \le z \le \delta_{o}) : \text{overlap layer} \\ \kappa \hat{u}_{*}\delta_{o} & (\delta_{o} \le z) : \text{outer layer} \end{cases}$



Boundary condition $\begin{cases} u = 0(z = 0); u = u_b (z \to \infty) \\ u & \partial u / \partial z : \text{continuous at the two interfaces} \end{cases}$

> Very complicated analytical solutions with Bessel functions of complex variables

"Complex friction factor" $ilde{C}$

$$\begin{aligned} \tau_b &= \hat{\tau}_b e^{i(\theta + \varepsilon)} \equiv \rho \tilde{C} \hat{u}_b u_b, \ \tilde{C} &= \hat{C} e^{i\varepsilon} \\ \hat{\tau}_b &= \rho \hat{C} \hat{u}_b^2 \qquad \longrightarrow \hat{C} = f_w/2 \\ R &\equiv R_\delta / \sqrt{2} = \sqrt{R_a}, \ \hat{u}_b / (\sigma z_0) = a_w / z_0 = 30 (a_w / k_s) \end{aligned}$$

Approximated expressions:







Wave friction factor for smooth and rough turbulence (implicit & explicit)



(3) Riedel, Kamphuis & Brebner (1972,13th ICCE, 587)

Direct measurement of τ_{b} in an oscillatory flow tank using a shear meter



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Phase lead is small in case of turbulent flows, ~ 10^o

Phase difference between bed shear stress and free-stream velocity (Fredsøe & Deigaard, 1992)





(4) US CERC (1984):

$$\tau_{\rm w} = \frac{1}{2} \rho f_{\rm w} U_{\rm b}^2 \qquad f_{\rm w} = 2 \left(\frac{U_{\rm b} A_{\rm b}}{v}\right)^{-0.5}$$

$$A_{\rm b} = \frac{1}{2\sinh(2\pi d/L)}$$
$$U_{\rm b} = \frac{\pi H}{T\sinh(2\pi d/L)}$$

 A_b : maximum bottom wave orbital amplitude U_b : maximum bottom wave orbital velocity d: water depth

$$\tau_{w} = H\left[\rho \frac{\left(v\left(\frac{2\pi}{T}\right)^{3}\right)^{0.5}}{2\sinh\left(\frac{2\pi d}{L}\right)}\right]$$



Friction Factor in a Coexistent Wave-current Field

(1) Grant & Madsen (1979, J. Geo. Res., 84, 1979)

$$f_{cw} = \alpha f_c + (1 - \alpha) f_w, \quad \alpha = \frac{U_c}{\hat{u}_b + U_c}$$

for the collinear case

Logarithmic velocity distribution $f_c = 2\kappa^2 \left[\ln \left(\frac{z_{um}}{z_0} \right) \right]^{-2}$

 Z_{um} : where the current velocity *Uc* is expected $Z_0 = k_s/30$: where the current velocity is zero

The first work presents an analytical solution for the wave-current friction factor

(2) Tanaka & Shuto (1981, Coastal Eng. in Japan, 24, 105)

One-layer model for v_e

Orbital velocity amplitude at the outer edge of the boundary layer

$$\hat{\mathbf{u}}_b = \frac{\pi H}{T \sinh(2\pi h/L)} (\cos \alpha, \sin \alpha)$$



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Moving coordinate system with velocity $\bar{U}_c \cos lpha$

No apparent current in wave direction & Same *L* \longrightarrow wave celerity $C \rightarrow C - \overline{U}_c \cos \alpha \downarrow$; then, $T \rightarrow T' \uparrow$

$$L = CT = \left(C - \overline{U}_c \cos \alpha\right)T' \rightarrow \frac{1}{T'} = \frac{1}{L} \left(\frac{L}{T} - \overline{U}_c \cos \alpha\right)$$

$$\hat{\mathbf{u}}_{b} = \frac{\pi H}{L} \frac{L/T - \overline{U}_{c} \cos \alpha}{\sinh(2\pi h/L)} (\cos \alpha, \sin \alpha)$$

Rough turbulent (example)
Boundary layer equation $\frac{\partial u}{\partial t} = -\frac{1}{\rho} \frac{\partial p}{\partial x} + \frac{\partial (v_e \partial u/\partial z)}{\partial z}$ $\mathbf{u}(z,t) = \mathbf{u}_w(z,t) + \mathbf{U}_c(z)$ $\mathbf{U}(z,t) = \mathbf{U}_w(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_v(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_w(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_v(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_w(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_v(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_w(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}_v(z,t) + \mathbf{U}_v(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}(z,t) + \mathbf{U}(z,t)$ $\mathbf{U}(z,t) = \mathbf{U}(z,t)$ \mathbf{U}(z,t)$ </

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$$\implies u_c^{*2} = \frac{\kappa U_c \hat{u}_{cw}^*}{\frac{1}{1 - z_0/h} \ln\left(\frac{h}{z_0}\right) - 1} \cong \frac{\kappa U_c \hat{u}_{cw}^*}{\ln\left(\frac{h}{z_0}\right) - 1}$$

$$\begin{array}{c} \textcircled{1} \quad \Longrightarrow \quad \frac{\partial \left(\mathbf{u}_{w} - \mathbf{u}_{b} \right)}{\partial t} = \frac{\partial}{\partial z} \left(\kappa \hat{u}_{cw}^{*} z \frac{\partial \mathbf{u}_{w}}{\partial z} \right) \end{aligned}$$

Considering the motion is periodic in time $\mathbf{u}_b - \mathbf{u}_w \equiv \mathbf{v} = \hat{v}(z)e^{i\sigma t}$ $\hat{v}(z)$ complex function

$$-i\sigma\hat{v}e^{i\sigma t} = \kappa\hat{u}_{cw}^* \left[-\frac{d\hat{v}}{dz} - z\frac{d^2\hat{v}}{dz^2} \right] e^{i\sigma t} \quad \Rightarrow \quad z\frac{d^2\hat{v}}{dz^2} + \frac{d\hat{v}}{dz} - ic\hat{v} = 0; \ c \equiv \frac{\sigma}{\kappa\hat{u}_{cw}^*}$$

Introduce a new complex variable

 $\xi \equiv 2\sqrt{cz}e^{-i\pi/4}$

$$\Rightarrow \qquad \xi^2 \frac{d^2 \hat{v}}{d\xi^2} + \xi \frac{d \hat{v}}{d\xi} + \xi^2 \hat{v} = 0 \rightarrow \frac{d^2 \hat{v}}{d\xi^2} + \frac{1}{\xi} \frac{d \hat{v}}{d\xi} + \hat{v} = 0$$

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Bessel's differential equation of the zero-th order



Bessel's differential equation of the γ -th order

$$\frac{d^2 y}{dx^2} + \frac{1}{x}\frac{dy}{dx} + \left(1 - \frac{\gamma^2}{x^2}\right)y = 0$$

0-th order Bessel's D.E. \implies General solution $\hat{v} = C_1 J_0(\xi) + C_2 N_0(\xi)$

$$J_{\gamma} = \sum_{n=0}^{\infty} \frac{(-1)^n}{n! \Gamma(\gamma + n + 1)} \left(\frac{x}{2}\right)^{2n+\gamma}$$
 First kind of Bessel function $N_{\gamma} = \frac{\cos \gamma \pi J_{\gamma} - J_{-\gamma}}{\sin \gamma \pi}$

$$\Gamma(\alpha) = \int_0^{\infty} x^{\alpha - 1} e^{-x} dx \ (\alpha > 0)$$
 Gama function Second kind of Bessel function

B.C.
$$\begin{cases} \text{No-slip} & \mathbf{u}_{w} = 0 \, (z = z_{0}) \to \hat{v} = \hat{\mathbf{u}}_{b} \left(\xi = \xi_{0} \right) \\ \text{Shear-free} & \mathbf{\tau}_{w} = 0 (z = h) \to d\hat{v}/d\xi = 0 \left(\xi = \xi_{h} \right) \end{cases} \\ J_{0}^{\prime} = -J_{1} \\ N_{0}^{\prime} = -N_{1} & \Longrightarrow \begin{cases} \hat{\mathbf{u}}_{b} = C_{1}J_{0}(\xi_{0}) + C_{2}N_{0}(\xi_{0}) \\ 0 = C_{1}J_{1}(\xi_{h}) + C_{2}N_{1}(\xi_{h}) \end{cases} \to C_{1} = \frac{\hat{\mathbf{u}}_{b}}{M}N_{1}(\xi_{h}), C_{2} = -\frac{\hat{\mathbf{u}}_{b}}{M}J_{1}(\xi_{h}) \\ M = N_{1}(\xi_{h})J_{0}(\xi_{0}) - J_{1}(\xi_{h})N_{0}(\xi_{0}) \end{cases}$$

$$\mathbf{u}_{w} = \mathbf{u}_{b} - \mathbf{v} = \hat{\mathbf{u}}_{b} \Re \left\{ \left[1 - \frac{N_{1}(\xi_{h})J_{0}(\xi) - J_{1}(\xi_{h})N_{0}(\xi)}{N_{1}(\xi_{h})J_{0}(\xi_{0}) - J_{1}(\xi_{h})N_{0}(\xi_{0})} \right] e^{i\sigma t} \right\}$$
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Maximum bottom friction



$$\frac{\sigma z_0}{\hat{u}_{cw}^*} = \frac{\sqrt{2/f_{cw}}}{a_m/z_0} \rightarrow \text{RHS} = \text{Function of}\left(\overline{U}_c/\hat{u}_w, \alpha, h/z_0, a_m/z_0, f_{cw}\right)$$

$$\implies f_{cw} = f_{cw} \left(\overline{U}_c / \hat{u}_w, \alpha, h / a_m, a_m / z_0 \right)$$

For "Laminar" and "Smooth turbulent"

$$f_{cw} = f_{cw} \left(\overline{U}_c / \hat{u}_w, \alpha, h / a_m, R_a = \frac{\hat{u}_w a_m}{v} \right)$$

Wave-Current Friction Law Covering all Flow Regimes Tanaka & Thu (1993) and Tanaka & Sana (1996) Approximated explicit expressions for Tanaka & Shuto (1981)

$$\tau_{b\max} = \rho \hat{u}_{cw}^{*2} = \frac{\rho}{2} f_{cw} \hat{u}_{w}^{2} \quad \left(u_{w} = \hat{u}_{w} = \hat{u}_{b}, U_{c} = \overline{U}_{c} \right)$$
$$\alpha' = \cos^{-1} \left(\left| \cos \alpha \right| \right) \text{ (rad)} \left(0 \le \alpha' \le \pi/2, 0 \le \alpha \le 2\pi \right)$$



Laminar:

$$f_{cw(L)} = \left\{ f_{c(L)}^2 + 2f_{c(L)}f_{w(L)}\cos\alpha' + f_{w(L)}^2 \right\}^{1/2}$$

$$f_{c(L)} = \frac{6}{R_c} \left(\frac{U_c}{u_w}\right)^2, \ f_{w(L)} = \frac{2}{\sqrt{R_a}}, \ R_c = \frac{U_c h}{v}, \ R_a = \frac{u_w a_m}{v}, \ \varepsilon_{(L)} = \pi/4$$

Smooth Turbulent: $(U_c/u_w) \le 5.0$

$$f_{cw(S)} = f_{c(S)} + 2\sqrt{f_{c(S)}\beta_{(S)}f_{w(S)}} \cos \alpha' + \beta_{(S)}f_{w(S)}$$

$$f_{c(S)} = \exp\left\{-7.60 + 5.98R_c^{-0.0977}\right\} \left(\frac{U_c}{u_w}\right)^2$$

$$f_{w(S)} = \exp\left\{-7.94 + 7.35R_a^{-0.0748}\right\}$$

$$\beta_{(S)} = \frac{1 + 0.871R_c^{-0.0362}f_{c(S)}^{0.177}(2\alpha'/\pi)^{2.5}}{1 + 5.04R_c^{-0.0303}f_{c(S)}^{0.379}}$$

$$\varepsilon_{(S)} = 0.74\xi_{(S)}^{0.153}\frac{1 + 0.00279\xi_{(S)}^{-0.357}}{1 + 0.127\xi_{(S)}^{0.563}}$$

$$\xi_{(S)} = 0.111/\left\{\kappa f_{cw(S)}R_a/2\right\}$$

Rough Turbulent:

$$\begin{split} f_{cw(R)} &= f_{c(R)} + 2\sqrt{f_{c(R)}\beta_{(R)}f_{w(R)}}\cos\alpha' + \beta_{(R)}f_{w(R)} \\ f_{c(R)} &= \frac{2\kappa^2}{\left[\ln\left(h/z_0\right) - 1\right]^2} \left(\frac{U_c}{u_w}\right)^2 \\ f_{w(R)} &= \exp\left\{-7.53 + 8.07 \left(\frac{a_m}{z_0}\right)^{-0.1}\right\} \\ \beta_{(R)} &= \frac{1 + 0.863\beta_1 \exp\left(-1.43\beta_1\right) \left(2\alpha'/\pi\right)^2}{1 + 0.769\beta_1^{0.83}} \\ \beta_1 &= \frac{1}{\ln\left(h/z_0\right) - 1} \frac{U_c}{u_w}, \quad z_0 = \frac{k_s}{30} \\ \varepsilon_{(R)} &= 0.74\xi_{(R)}^{0.153} \frac{1 + 0.00279\xi_{(R)}^{-0.357}}{1 + 0.127\xi_{(R)}^{0.563}}, \quad \xi_{(R)} = \frac{1}{2} \left| \frac{\kappa_s \sqrt{\frac{f_{cw(R)}}{2} \frac{a_m}{z_0}}}{\frac{1}{2} \frac{1}{2}} \right| \\ \end{split}$$

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All Flow Regimes

$$\begin{split} f_{cw} &= f_2 \left\{ f_1 f_{cw(L)} + \left(1 - f_1\right) f_{cw(S)} \right\} + \left(1 - f_2\right) f_{cw(R)} \\ f_1 &= \exp\left\{ -0.0513 \left(\frac{R_{cw}}{2.5 \times 10^5}\right)^{4.65} \right\}, \quad f_2 = \exp\left\{ C_1 \left(\frac{R_{cw}}{R_1}\right)^{C_2} \right\} \\ C_1 &= -0.0101 - 0.3469 \gamma^{0.2}, \quad C_2 = 2.06 - 1.09 \gamma^{0.05} \\ R_{cw} &= 500 R_c + R_a, \quad R_1 = 0.501 \zeta^{1.15}, \quad \zeta = 350 \gamma \frac{h}{z_0} + \left(1 - \gamma\right) \frac{a_m}{z_0}, \quad \gamma = \frac{U_c / u_w}{1 + U_c / u_w} \\ \varepsilon &= f_2 \left\{ f_1 \varepsilon_{(L)} + \left(1 - f_1\right) \varepsilon_{(S)} \right\} + \left(1 - f_2\right) \varepsilon_{(R)} \end{split}$$

Transitions between flow regimes

Laminar to Turbulent $2.5 \times 10^5 < R_{_{CW}} < 6.0 \times 10^5$

Smooth to Rough $0.501\zeta^{1.15} < R_{cw} < 7.00\zeta^{1.15}$





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Wave-current friction factor for various U_c/u_w

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Wave-current friction factor for various angle

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(3) Soulsby(1997)

Maximum bottom shear stress during a wave cycle

$$\tau_b = \sqrt{\left(\tau_m + \tau_w |\cos\lambda|\right)^2 + \left(\tau_w |\sin\lambda|\right)^2}$$

 λ : the angle between wave and current

Mean bottom shear stress

$$\tau_m = \tau_c \left[1 + 1.2 \left(\frac{\tau_w}{\tau_c + \tau_w} \right)^{3.2} \right]$$

$$\tau_w = \frac{1}{2} \rho_w f_w U_w^2, \qquad \tau_c = \rho u_*^2 \quad \frac{u}{u_*} = \frac{1}{\kappa} \ln\left(\frac{z}{z_o}\right)$$

$$f_{w} = \max\{f_{wr}, f_{ws}\}, f_{wr} = 0.237r^{-0.52}, f_{ws} = B \operatorname{Re}_{w}^{-N},$$

$$r = A / k_{s}, \operatorname{Re}_{w} = U_{w}A / v, A = U_{w}T / (2\pi), U_{w} = \frac{\pi H}{T \sinh(2\pi d / L)}$$

B = 2, N = 0.5 for $\text{Re}_{w} \le 5 \times 10^{5}; B = 0.0521, N = 0.187$ for $\text{Re}_{w} > 5 \times 10^{5}$

A : maximum bottom wave orbital amplitude

U_w: maximum bottom wave orbital velocity

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In a Coexistent Wave-current Field

- 如果作用于光滑床面上潮流和波浪的速度 够小且水流为层流,则床面切应力的合力 是潮流与波浪各自所产生的切应力的简单 线性叠加;
- 若潮流与波浪强度较大,水流就会产生紊动,紊流产生在潮流与波浪的边界层内,则切应力的合力是非线性叠加的。



作业二

• At the nearshore of Zhoushan Island, wave and current that co-exist may lead to sediment resuspension. The flow can be assumed as turbulence over a rough bottom. The angle between the wave and current is 30° and water depth is 5 m. The current speed at z=0.1 m is 0.2 m/s, and k_s is 0.15 m. The wave height is 2 m and wave period is 5 s. Please estimate the maximum total bottom shear stress by using Kajiura (1968)'s explicit method, US CERC (1984), and Soulsby (1997) methods. The critical shear stress for the bottom sediment is 0.1 N/m². Can the wave and current induced bottom shear stress initiate the bottom sediments?

Due 2018/5/22 (in class).







